

Traffic Engineering Study

# Willow, Bridge, Depot, and Prospect Streets Project

# Winchester, Connecticut

#### PREPARED FOR

Town of Winchester 338 Main Street Winsted, CT 06098

PREPARED BY



100 Great Meadow Road Suite 200 Wethersfield, Connecticut 06109 860.807.4300

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# Introduction

Vanasse Hangen Brustlin, Inc. (VHB) has been retained by the town of Winchester to conduct a traffic engineering study for the intersections of Bridge Street at Willow Street, Prospect Street, and Depot Street in Winchester, Connecticut. The purpose of this engineering study is to evaluate existing conditions and to develop alternative conceptual plans to improve traffic operating conditions, pedestrian mobility, access to the adjacent development, and safety. An additional goal of this project is to identify options to increase the quantity of on-street parking to support adaptive reuse of the adjacent old mill building and encourage future economic development in the area.

This document provides a detailed description of the study methodology and recommendations.

2 Introduction



1

# **Existing Conditions**

The first phase of this project consisted of a thorough evaluation of existing conditions in the project area. As part of this existing conditions assessment, VHB has reviewed the physical characteristics of the roadways and intersections, collected traffic count data, evaluated crash history, and conducted capacity analyses to evaluate current traffic operating conditions. A summary of the existing conditions assessment is presented in the following section.

#### **Description of Existing Roadways**

The following section provides a description of the physical characteristics of the existing roadways and intersections in the project area.

#### **Study Roadways**

#### **Bridge Street**

Bridge Street is a two-lane (one lane in each direction) roadway with a posted speed limit of 25 miles per hour. Bridge Street begins at the intersection of Main Street (Route 44/183) in the north and continues south through the project area providing access to a residential neighborhood. Bridge Street is classified by CTDOT as a collector roadway north of Prospect Street and as a local road south of Prospect Street. A double yellow line is striped on Bridge Street between Main Street and Willow Street. There are no pavement markings on Bridge Street south of Willow Street. Bridge Street has a very steep slope (over 15-percent) south of Willow Street. There are sidewalks along both sides of Bridge Street north of Prospect Street and no sidewalks further south.

#### **Prospect Street**

Prospect Street is a two-lane (one lane in each direction) collector roadway (as classified by CTDOT) with a posted speed limit of 25 miles per hour. Prospect Street begins at the intersection with Bridge Street and continues west, parallel to Main Street (Route 44/183), and ends at the intersection with Lake Street. A painted stop-



bar and double yellow centerline is provided on Prospect Street at the intersection with Bridge Street. There are sidewalks on the north side of Prospect Street.

#### Willow Street

Willow Street is a two-lane (one lane in each direction) local roadway with a posted speed limit of 25 miles per hour. Willow Street begins at the intersection with Bridge Street and continues east to Rowley Street. A painted stop-bar and crosswalk are provided on Willow Street at the intersection with Bridge Street. On-street parking is permitted on both sides of Willow Street in the project area. There are sidewalks along both sides of Willow Street in the vicinity of Bridge Street. The sidewalk along the north side of Willow Street ends approximately 40-feet east of Bridge Street, and the sidewalk along the south side ends approximately 170-feet east of Bridge Street.

#### **Depot Street**

Depot Street Avenue is a two-lane (one lane in each direction) local roadway with a speed limit of 25 miles per hour. Depot Street begins at its intersection with Bridge Street in the west and runs approximately 200-feet to the east, where the road bends to the right and becomes Charles Street. There are no sidewalks along Depot Street.

#### Study Intersections

#### **Bridge Street at Willow Street**

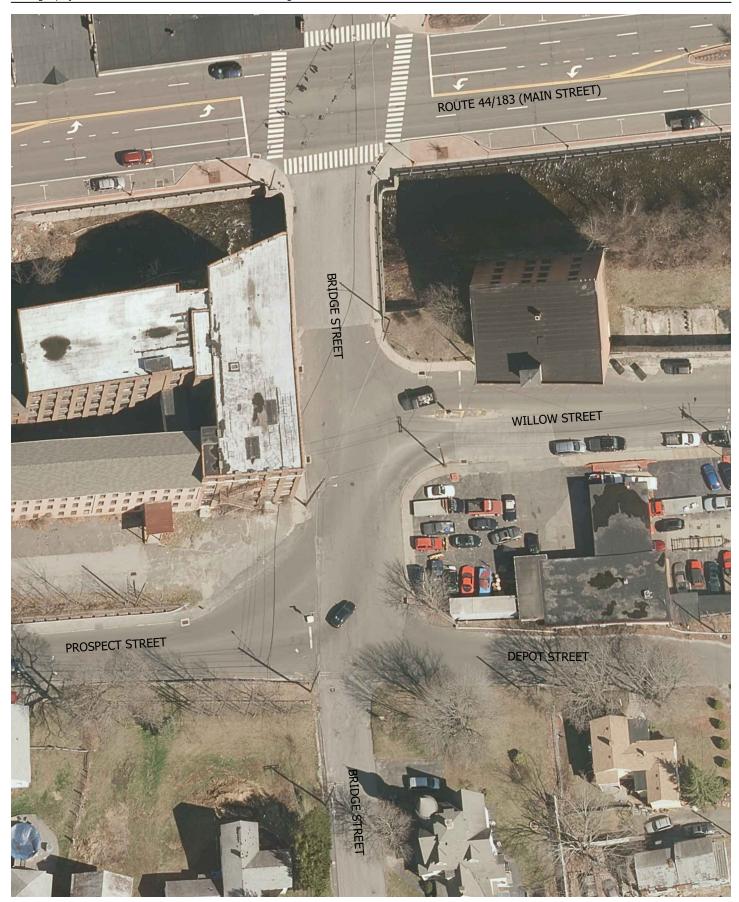
Bridge Street and Willow Street form a three-leg intersection approximately 100-feet south of Main Street (Route 44/183). The northbound and southbound Bridge Street approaches operate under free-flow conditions, and the westbound Willow Street approach is stop controlled. All three approaches at the intersection consist of a single lane. A striped crosswalk is provided across Willow Street. There are no crosswalks provided across Bridge Street.

#### **Bridge Street at Prospect Street and Depot Street**

Bridge Street is intersected by Prospect Street from the west and Depot Street from the east to form a four-leg intersection with atypical geometry approximately 75-feet south of Willow Street. The northbound and southbound Bridge Street approaches operate under free-flow conditions. The eastbound Prospect Street and westbound Depot Street approaches are stop controlled. All four approaches consist of a single lane. No crosswalks are provided at this intersection.

A driveway for the mill building is located at the northwest corner of the intersection. This northwestern curb has a large radius, which enables motorists to travel at higher speeds turning right from Bridge Street onto Prospect Street. The steep slope on Bridge Street continues through this intersection before slightly leveling off, creating a hump with a steep cross slope for motorists entering the intersection from Prospect Street or Depot Street.

An aerial view of the project area is shown on *Figure* 1.



Source: Capitol Region Council of Governments. (2016). 2016 Aerial imagery. Retrieved from http://cteco.uconn.edu/data/flight2016/index.htm.





#### FIGURE 1



#### **Crash Data**

Crash records within the project area were obtained from the University of Connecticut, Connecticut Crash Data Repository (CTCDR) for the most recent three-year period available, from January 2016 to December 2018. Each collision case report contained the following information: police case number, date, time, location, collision type, collision severity, number of vehicles involved, travel direction of each vehicle involved, weather/lighting conditions, and contributing factor (if available). It should be noted that only collisions that result in death, injury, or property damage in excess of \$1,000 are required to be reported.

Based on this data, five crashes were reported in the study area during the three-year analysis period. One crash involved a vehicle hitting a fixed object on Prospect Street. Two crashes were rear-end collisions involving vehicles traveling westbound on Willow Street towards Bridge Street. The other two crashes were angle collisions within the intersection of Bridge Street at Willow Street. There were no apparent injuries documented in any of the crashes reported in the study area.

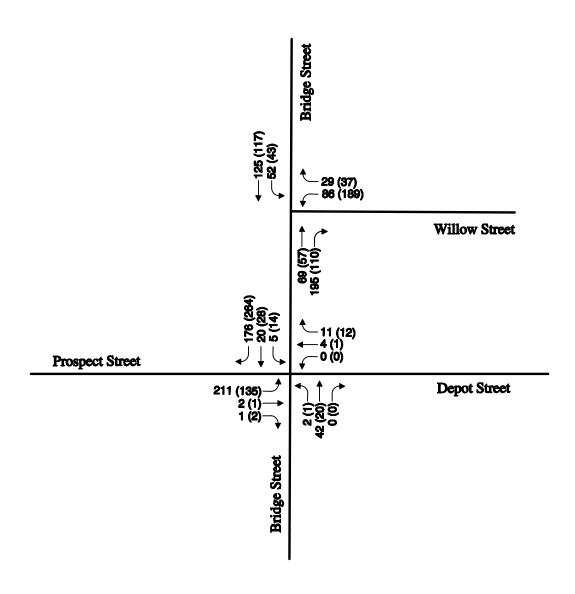
Five reported crashes over three years is typically considered a low crash rate for this type of intersection with unusual geometry. Although this data contains a relatively low frequency of crashes over the past three years, it should be noted that Town staff has indicated that "near misses" occur frequently in this area. Additionally, the mill building was vacant or only partially occupied during the three-year analysis period. Additional traffic generated by the anticipated re-use of this building or other nearby development could affect the frequency of crashes in the future.

## **Existing Traffic Volumes**

To assess existing traffic conditions in the study area, peak hour manual turning movement traffic counts were recorded on Wednesday, December 5, 2018 during the typical morning (7am-9am) and evening (4pm-6pm) peak traffic periods.

The peak hour turning movement volumes are summarized on *Figure 2*. As shown on this figure, the heaviest traffic movement through these intersections involves motorists making an 'S' movement traveling between Prospect Street and Willow Street. The complete traffic count data is included in the appendix.

Only three pedestrians were observed crossing the road in the study area during the four hours in December when the traffic counts were collected. However, Town staff has indicated that there is much higher pedestrian activity during the summer. In particular, the Town noted that children frequently walk through the project area during the summer to travel between Main Street and the baseball fields further east on Willow Street. Additionally, the Town plans to extend a multi-use trail, which currently ends at the western limits of the parking lot for the old mill building, through the project area and to the east along Willow Street. This trail crossing would increase pedestrian traffic crossing Bridge Street.



<u>KEY</u>

XX - AM PEAK HOUR TRAFFIC VOLUMES (XX) - PM PEAK HOUR TRAFFIC VOLUMES







#### **Intersection Capacity Analysis**

To assess the quality of traffic flow in the study area during the peak periods, intersection capacity analyses were conducted for the weekday morning and weekday evening peak traffic periods using the traffic count data discussed in the previous section. The intersection capacity analyses were conducted based on the evaluation criteria contained in the <u>2010 Highway Capacity Manual</u> (HCM) using Synchro<sup>TM</sup> software (Version 9). Synchro software is widely used by traffic engineering professionals and is consistent with the procedures in the HCM.

Capacity analyses results are reported using a variety of performance measures, including "Level of Service" (LOS) and vehicle queue length. The level of service designation is based on the average delay experienced by a vehicle traveling through the intersection. Similar to a report card, LOS designations are letter based, ranging from A to F, with LOS A representing the best operating condition (lowest vehicle delays) and LOS F representing the worst operating condition (highest vehicle delays). LOS D or better is typically considered tolerable, whereas LOS E and F represents overly congested conditions.

For unsignalized intersections with stop-control on the side street approaches (such as Bridge Street at Willow Street, Depot Street, and Prospect Street), the analysis assumes that through and right-turning movements on the main street (Bridge Street) are unimpeded by side street traffic. As such, LOS and delay are reported only for left-turns from the main street and for all movements from the side street.

Vehicle queue lengths are typically reported in terms of the 95<sup>th</sup> percentile vehicle queue. The 95<sup>th</sup> percentile queue is the queue length (in number of vehicles) which is expected to be exceeded only 5-percent of the time during the peak period analyzed. The 95<sup>th</sup> percentile queue is generally considered the maximum queue for design purposes.

The results of the capacity analysis are summarized in *Table 1*.

<sup>&</sup>lt;sup>1</sup> <u>Highway Capacity manual 2010</u>; Transportation Research Board, National Research Council, Washington, DC (2010).



Table 1
Capacity Analysis Summary

Intersection / Time Period	Lane Group	LOS1	Dolay?	Queue <sup>3</sup>
intersection / Time i enou	Lanc Group	LUS	Delay <sup>2</sup>	Queue
Bridge Street at Willow Street				
Weekday Morning Peak Hour	- Westbound Willow Street	В	13.9	0.9
	- Southbound Left-turns	Α	7.3	0
Weekday Evening Peak Hour	- Westbound Willow Street	С	18.9	2.8
	- Southbound Left-turns	Α	8.3	0
Bridge Street at Prospect Street & Depot Street				
Weekday Morning Peak Hour	- Eastbound Prospect Street	В	13.3	1.7
	- Westbound Depot Street	Α	9.5	0.1
	- Northbound Left-turns	Α	7.8	0
	- Southbound Left-turns	Α	7.3	0
Weekden Franken Beek Henr	Footh and David at Otrock	<b>D</b>	40.4	0.0
Weekday Evening Peak Hour	- Eastbound Prospect Street	В	12.4	0.9
	- Westbound Depot Street	A	9.6	0.1
	- Northbound Left-turns	Α	8.0	0
	- Southbound Left-turns	Α	7.3	0

Source: Vanasse Hangen Brustlin, Inc. using Synchro 9.0 software.

As shown in the table above, Bridge Street, Prospect Street, and Depot Street all operate at LOS B or better conditions during the weekday morning and evening peak traffic periods. Willow Street operates at LOS B during the weekday morning peak traffic period and LOS C during the weekday evening peak traffic period.

This analysis indicates that the intersections are currently operating well below capacity with low delays during the peak traffic periods.

<sup>1</sup> Level of service

<sup>2</sup> Average control delay of the critical approach in seconds per vehicle

<sup>3 95</sup>th percentile queue in number of vehicles



#### **Existing Conditions Summary**

The following is a summary of the key issues identified through this existing conditions assessment:

- > The highest volume traffic movement in the study area involves motorists traveling between Willow Street and Prospect Street. These two streets are offset by approximately 75-feet, which results in a high volume of traffic making an 'S' movement through the intersections. Additionally, the Prospect Street intersection has an atypical geometry with a very large radius on the northwest corner, which permits motorists to travel at higher speeds turning right onto Prospect Street.
- ➤ A wide driveway for the mill building is located on the northwest corner of Bridge Street and Prospect Street. The location of this driveway is problematic, because it is located within the intersection, and the sight distance for motorists exiting this driveway is significantly limited by the existing building to the left.
- ➤ Bridge Street has a very steep slope at the intersection with Prospect Street. This excessive slope limits the available options to reconfigure this intersection. Installation of "Stop" or "Yield" signs on Bridge Street could create unsafe conditions as motorists would have difficulty stopping in wet or icy conditions. Additionally, geometric modifications to the intersection configuration could require significant earthwork and installation of a retaining wall to address the steep grades.
- There are currently no crosswalks provided across Bridge Street within the study area. Additionally, the atypical intersection configuration at Prospect Street results in long and awkward crossing maneuvers. The need for a safe and convenient crossing across Bridge Street is important as the Town moves forward with plans to extend a multi-use trail from the old mill property west of Bridge Street to the east along Willow Street.
- There is limited parking available in the vicinity of the old mill building. The town has expressed a desire to increase the quantity of on-street parking in the area to support adaptive re-use of an old mill building and encourage future economic development.
- The capacity analysis results indicate that the study intersections do not experience excessive delays during the peak traffic periods. Therefore, increasing capacity is not needed, and future improvements should be developed to prioritize the other issues noted above, particularly safety and pedestrian accessibility.



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# **Recommended Improvements**

VHB developed multiple conceptual improvement plans to address the issues identified in the previous section. Based on discussions with Town staff, two of these plans were selected as preferred alternatives. The first alternative is a "short-term" plan that could be constructed at moderate cost with minimal right-of-way impacts. The other alternative selected is a "long-term" plan intended for future planning purposes, as it would require more significant costs and property acquisition.

The preferred alternatives are discussed in the following section. The concept plans that were rejected by the Town are included in the Appendix.

## **Short-term Improvements Plan**

The short-term conceptual improvement plan is depicted on *Figure 3*. This plan involves a realignment of Prospect Street, Bridge Street, Depot Street, and the mill driveway to form a more traditional four-leg intersection with the western leg of Prospect Street and the northern leg of Bridge Street aligned to effectively become the new through movement. Additionally, the southern leg of Bridge Street will be realigned to intersect with Depot Street immediately east of the Prospect Street/mill driveway intersection. This realignment of Bridge Street may potentially require an easement or partial taking of the property on the southeast corner. However, right-of-way information is not currently available to evaluate these potential impacts.

A new crosswalk is provided across the northern leg of Bridge Street. VHB recommends installing stop signs on all four approaches to the intersection to improve safety for pedestrians. Additionally, all-way stop control mitigates the lack of sight distance from the mill driveway, since motorists exiting the driveway would only need to be able to see up to the opposing stop bar.

Under this plan, Bridge Street is converted to one-way southbound traffic flow between Maple Street and a residential driveway to the north. This one-way street conversion is intended to address safety concerns caused by motorists traveling northbound down the hill on Bridge Street and being required to stop at a stop sign

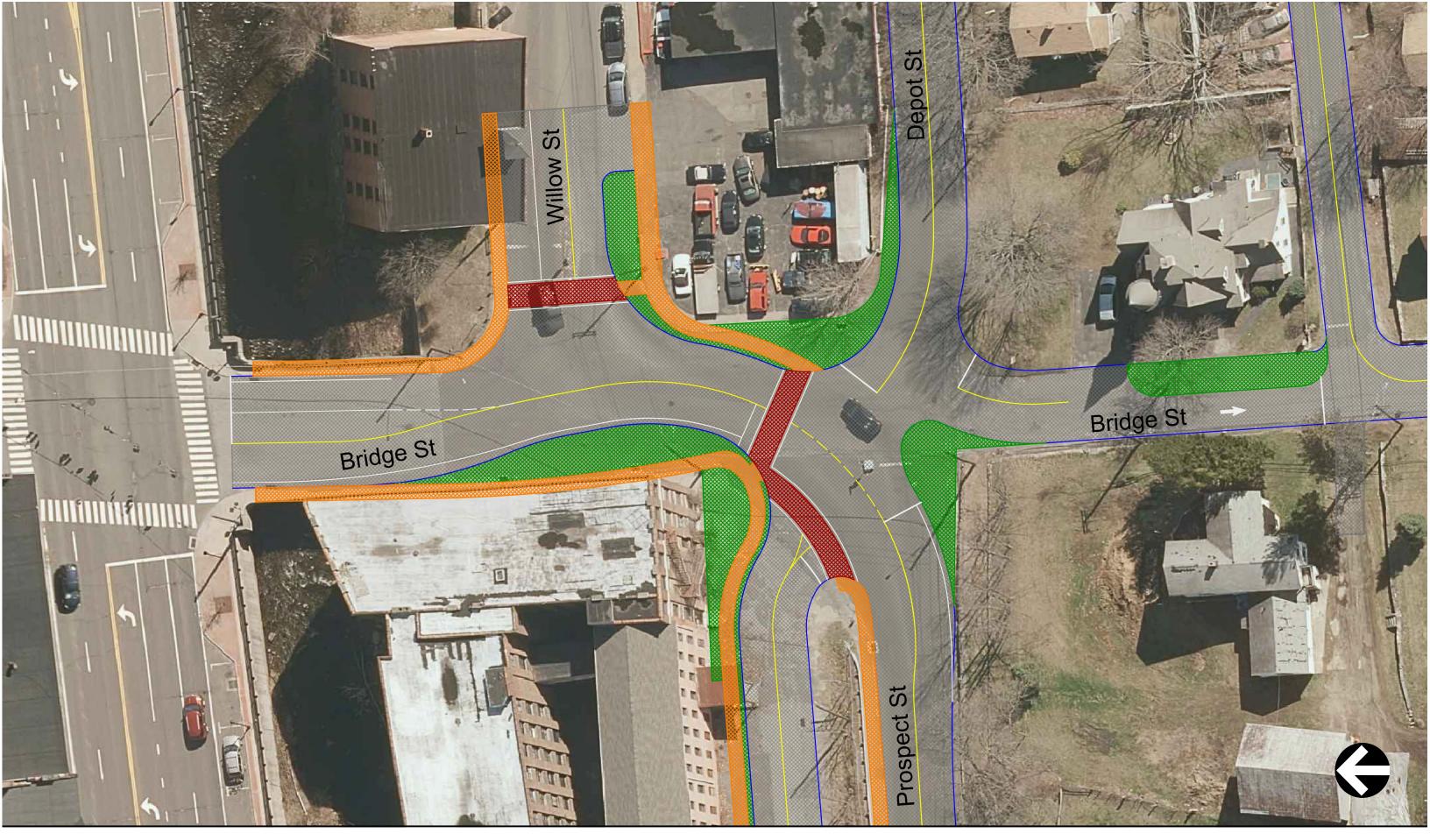


on a steep hill. Two-way traffic flow is retained between the residential driveway and Depot Street to maintain access to the street network for the residence.

It should be noted that closing the southern leg of Bridge Street was also investigated as a short-term improvement. Closing the southern leg of Bridge Street would allow more significant improvements by regrading the intersection to reduce the slopes. However, the Fire Department indicated that closing Bridge Street would divert Fire trucks through residential neighborhoods with intersections that are not adequately designed to accommodate fire truck turning movements. This would require additional geometric improvements to intersections that are beyond the scope of this project. Therefore, the Town has indicated that closing the southern leg of Bridge Street is not feasible as a short-term improvement.

#### **Construction Cost Estimate**

VHB has completed a planning level construction cost estimate for these improvements and estimates the order of magnitude construction cost to be approximately \$350,000. This estimate includes incidentals and contingencies but excludes potential right of way costs.





Willow, Bridge, Depot, and Prospect Street Project Winchester, CT



Figure 3 Short-Term Improvements August 2019





#### **Long-term Improvements Plan**

The long-term conceptual improvement plan is depicted on *Figure 4*. This plan includes a similar intersection realignment of Bridge Street and Prospect Street as the short-term plan, except that the southern leg of Bridge Street is closed entirely. Additionally, this plan includes a realignment of Willow Street onto the current alignment of Depot Street. The existing western portion of Willow Street is converted to one-way eastbound with the addition of on-street parking spaces. Although right-of-way information is not currently available, this long-term plan would require significant acquisition of private property.

This plan offers significant operational benefits over the short-term improvements. Realigning Willow Street opposite of Prospect Street reduces delays for motorists traveling between Willow Street and Prospect Street. This realignment also provides additional safety benefits by reducing potential conflicts associated with traffic turning onto Bridge Street from Willow Street. Additionally, the existing western portion of Willow Street can be reconfigured to provide additional parking.

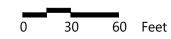
#### **Construction Cost Estimate**

VHB has completed a planning level construction cost estimate for these improvements and estimates the order of magnitude construction cost to be approximately \$750,000. This estimate includes incidentals and contingencies but excludes potential right of way costs.





Willow, Bridge, Depot, and Prospect Street Project Winchester, CT







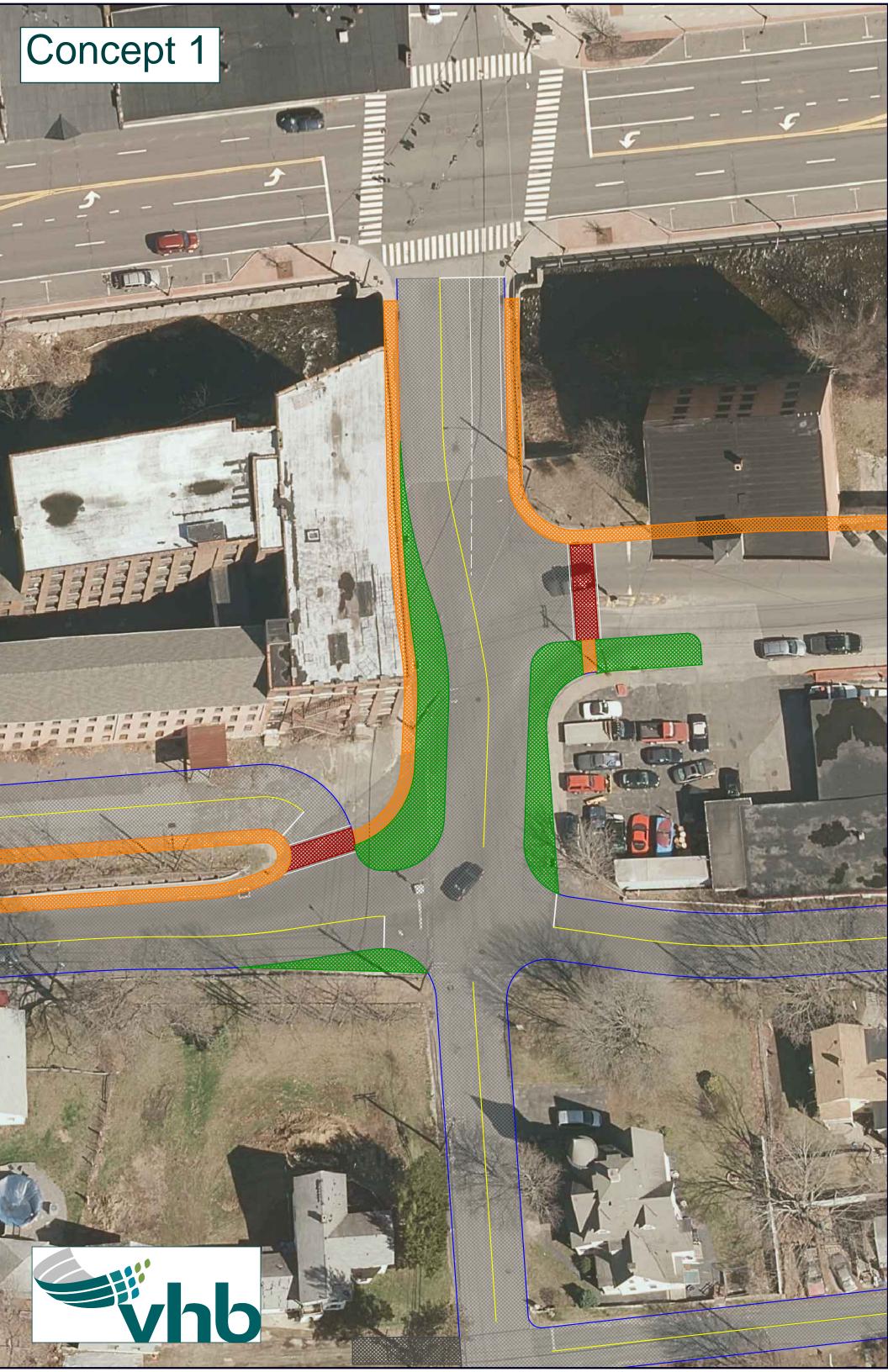
# Appendix

**Rejected Conceptual Improvement Plans** 

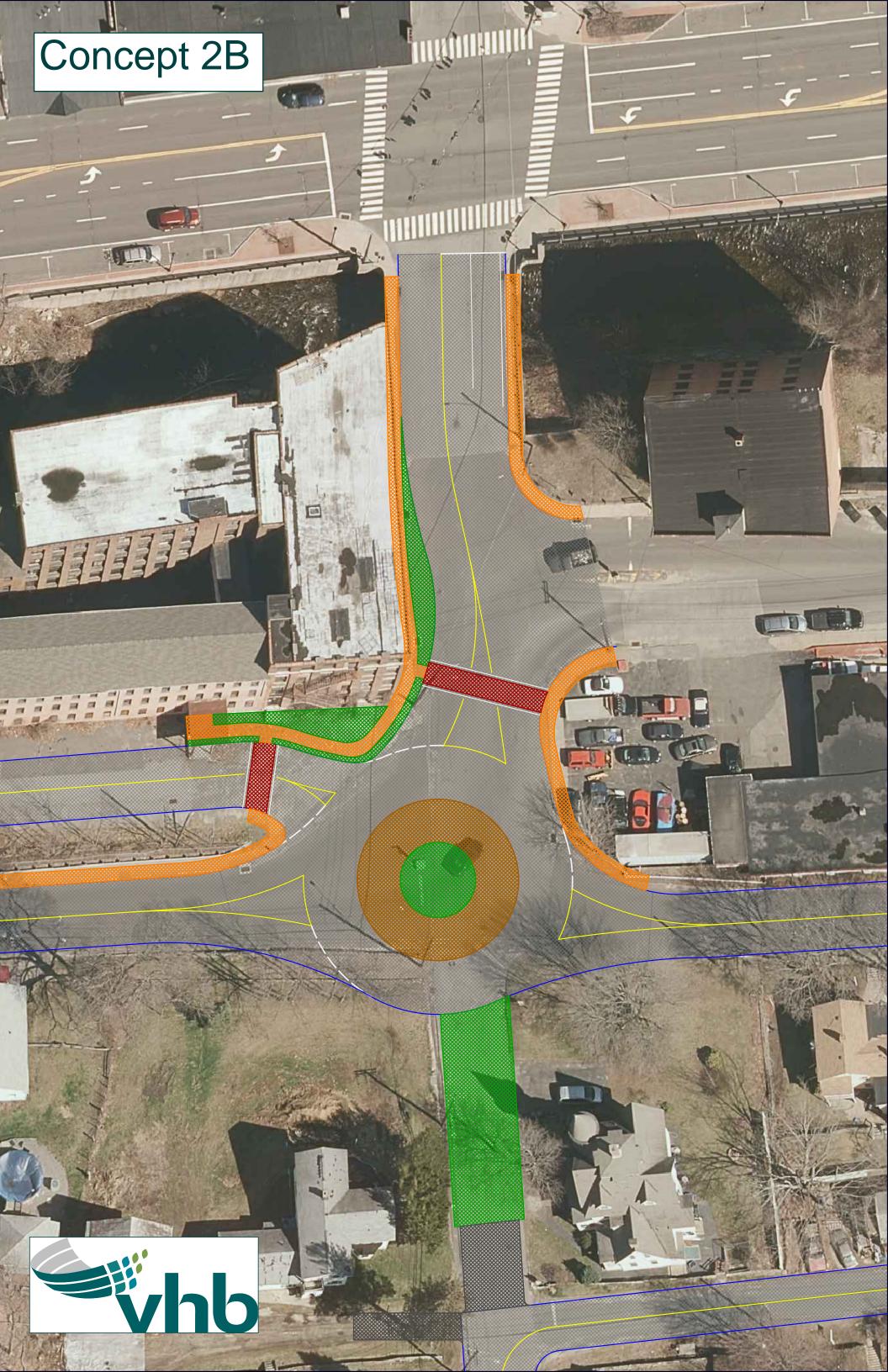
**Traffic Count Data** 

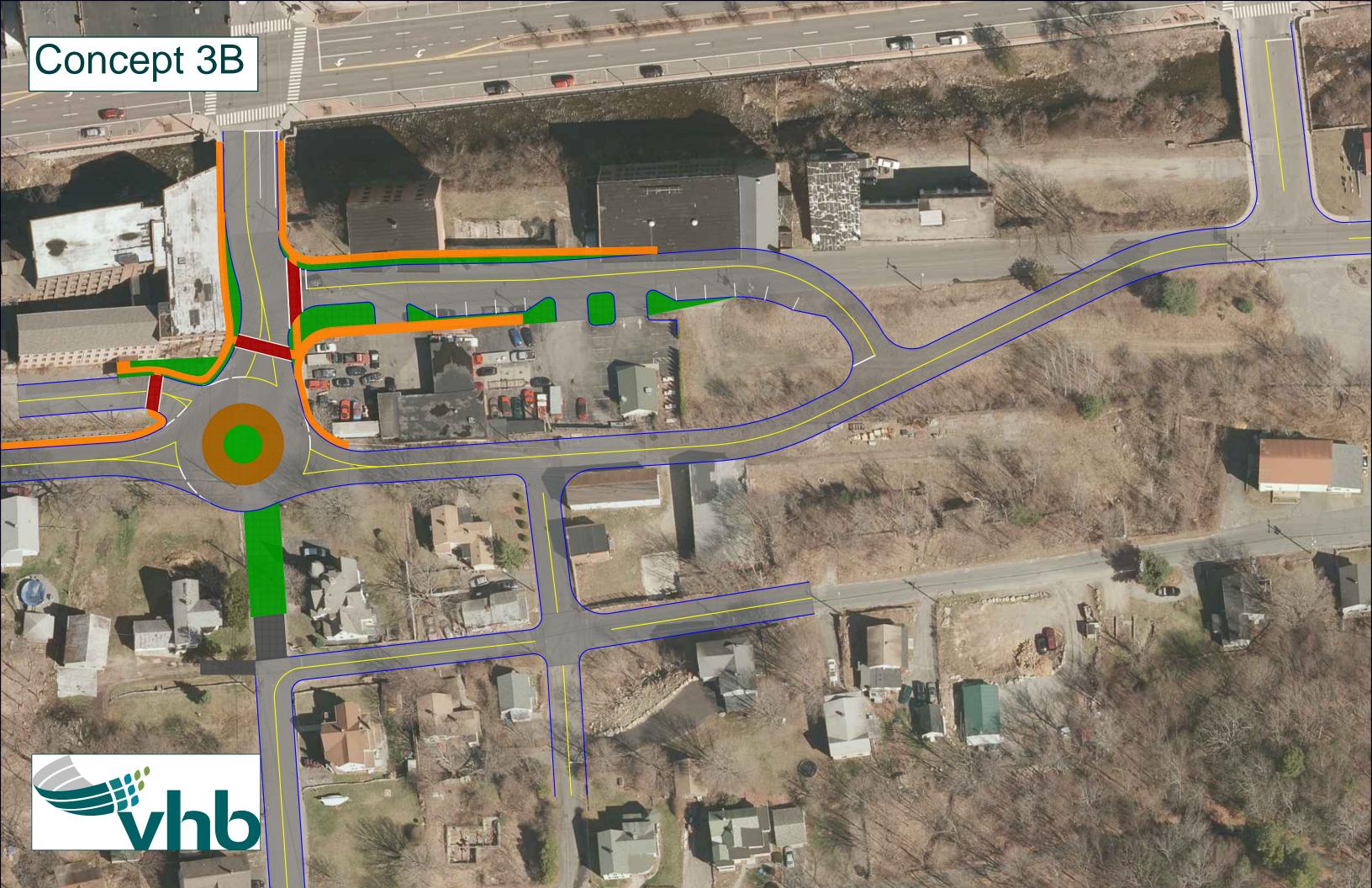
**Intersection Capacity Analysis** 

# **Rejected Conceptual Improvement Plans**

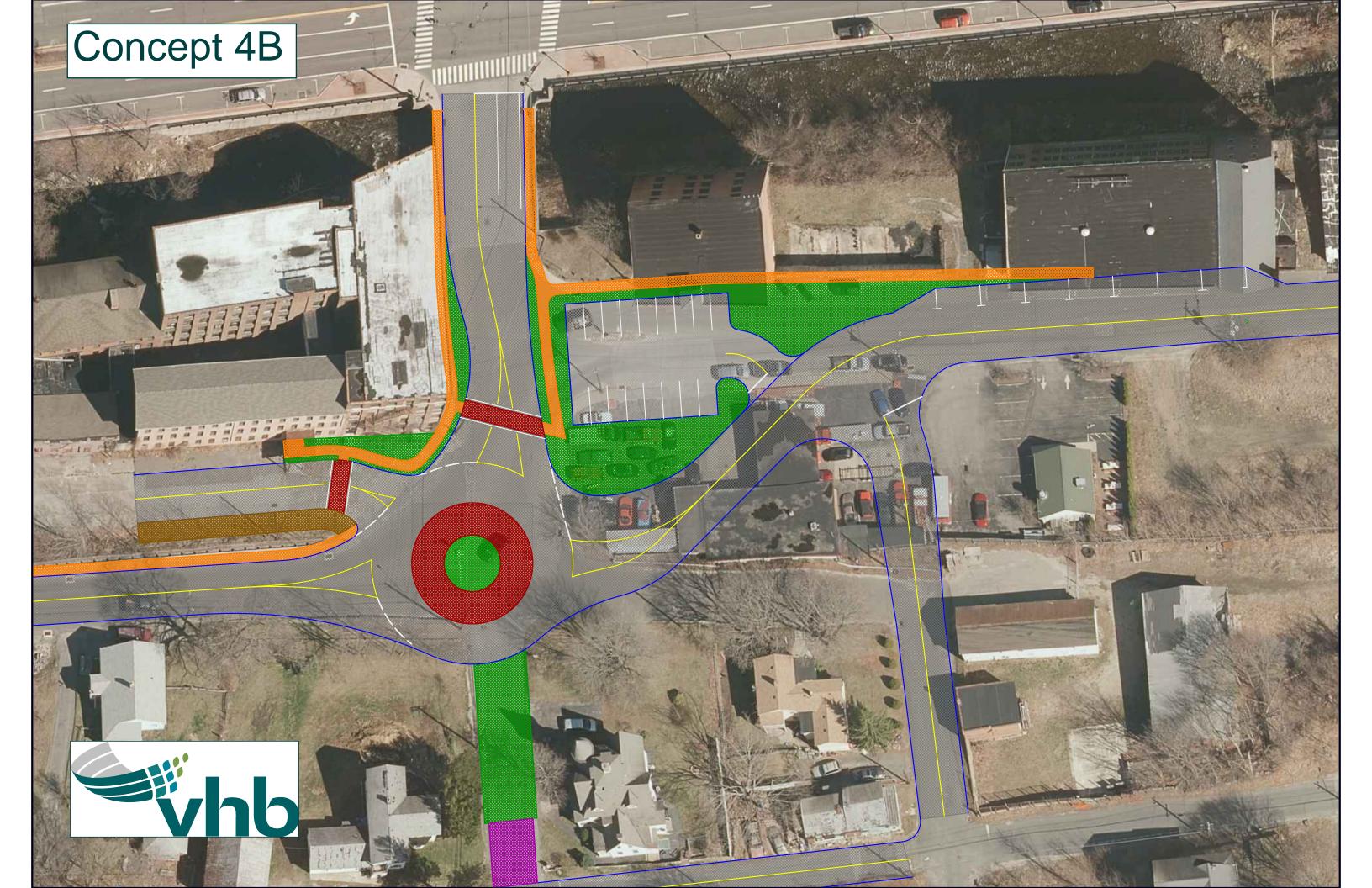












# **Traffic Count Data**

Kensington, Connecticut 06037 (860) 828-1693

Bridge Street at Prospect St/Willow St Winchester, Connecticut

File Name : 18222 Site Code : 18222 Start Date : 12/5/2018

Page No : 1

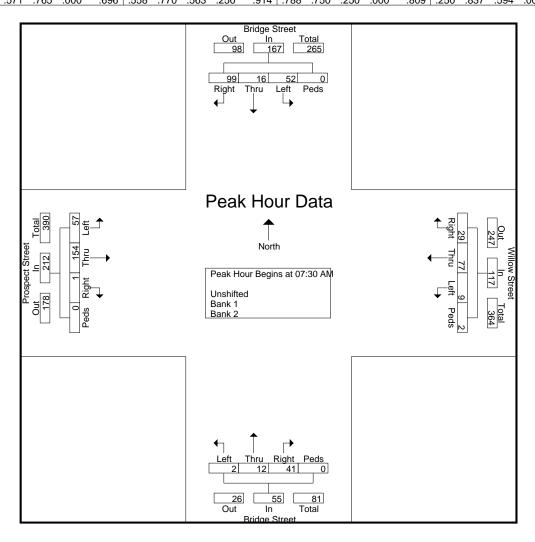
Groups Printed- Unshifted - Bank 1 - Bank 2

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Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	10	0	10	0	20	10	11	0	0	21	4	2	0	0	6	0	34	4	0	38	85
07:15 AM	11	2	10	0	23	15	13	1	0	29	6	3	1	0	10	0	31	11	0	42	104
07:30 AM	11	4	14	0	29	13	13	1	0	27	13	4	0	0	17	1	40	8	0	49	122
07:45 AM	25	1	16	0	42	8	22	2	0	32	9	3	0	0	12	0	28	7	0	35	121
Total	57	7	50	0	114	46	59	4	0	109	32	12	1	0	45	1	133	30	0	164	432
08:00 AM	27	4	5	0	36	7	17	4	2	30	12	2	2	0	16	0	46	24	0	70	152
08:15 AM	36	7	17	0	60	1	25	2	0	28	7	3	0	0	10	0	40	18	0	58	156
08:30 AM	16	3	7	0	26	5	13	0	0	18	3	3	0	0	6	0	27	18	0	45	95
08:45 AM	16	2	8	0	26	6	14	0	0	20	4	5	0	0	9	0	46	15	0	61	116
Total	95	16	37	0	148	19	69	6	2	96	26	13	2	0	41	0	159	75	0	234	519
Grand Total	152	23	87	0	262	65	128	10	2	205	58	25	3	0	86	1	292	105	0	398	951
Apprch %	58	8.8	33.2	0		31.7	62.4	4.9	1		67.4	29.1	3.5	0		0.3	73.4	26.4	0		
Total %	16	2.4	9.1	0	27.5	6.8	13.5	1.1	0.2	21.6	6.1	2.6	0.3	0	9	0.1	30.7	11	0	41.9	
Unshifted	140	22	77	0	239	54	122	10	2	188	58	25	2	0	85	1	280	100	0	381	893
% Unshifted																					
Bank 1	0	0	2	0	2	6	4	0	0	10	0	0	0	0	0	0	2	0	0	2	14
% Bank 1	0	0	2.3	0	0.8	9.2	3.1	0	0	4.9	0	0	0	0	0	0	0.7	0	0	0.5	1.5
Bank 2	12	1	8	0	21	5	2	0	0	7	0	0	1	0	1	0	10	5	0	15	44
% Bank 2	7.9	4.3	9.2	0	8	7.7	1.6	0	0	3.4	0	0	33.3	0	1.2	0	3.4	4.8	0	3.8	4.6

Kensington, Connecticut 06037 (860) 828-1693

> File Name : 18222 Site Code : 18222 Start Date : 12/5/2018

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07:30 AM	11	4	14	0	29	13	13	1	0	27	13	4	0	0	17	1	40	8	0	49	122
07:45 AM	25	1	16	0	42	8	22	2	0	32	9	3	0	0	12	0	28	7	0	35	121
08:00 AM	27	4	5	0	36	7	17	4	2	30	12	2	2	0	16	0	46	24	0	70	152
08:15 AM	36	7	17	0	60	1	25	2	0	28	7	3	0	0	10	0	40	18	0	58	156
Total Volume	99	16	52	0	167	29	77	9	2	117	41	12	2	0	55	1	154	57	0	212	551
% App. Total	59.3	9.6	31.1	0		24.8	65.8	7.7	1.7		74.5	21.8	3.6	0		0.5	72.6	26.9	0		
PHF	688	571	765	000	696	558	770	563	250	91/	788	750	250	000	മറമ	250	837	50/	000	757	883

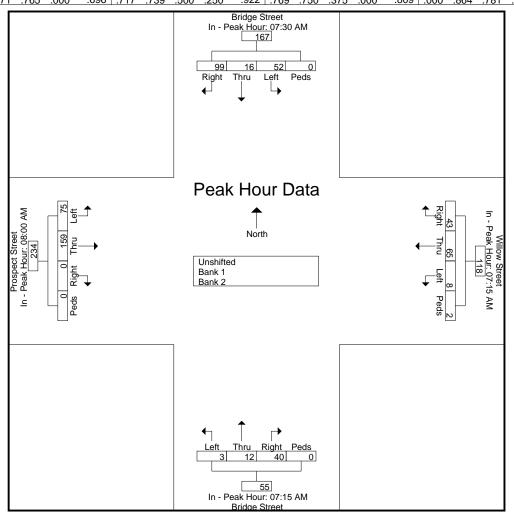


Kensington, Connecticut 06037 (860) 828-1693

File Name: 18222 Site Code: 18222 Start Date : 12/5/2018

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+15 mins.	25	1	16	0	42	13	13	1	0	27	13	4	0	0	17	0	40	18	0	58
+30 mins.	27	4	5	0	36	8	22	2	0	32	9	3	0	0	12	0	27	18	0	45
+45 mins.	36	7	17	0	60	7	17	4	2	30	12	2	2	0	16	0	46	15	0	61
Total Volume	99	16	52	0	167	43	65	8	2	118	40	12	3	0	55	0	159	75	0	234
% App. Total	59.3	9.6	31.1	0		36.4	55.1	6.8	1.7		72.7	21.8	5.5	0		0	67.9	32.1	0	
PHF	.688	.571	.765	.000	.696	.717	.739	.500	.250	.922	.769	.750	.375	.000	.809	.000	.864	.781	.000	.836



Kensington, Connecticut 06037 (860) 828-1693

Bridge Street at Prospect St/Willow St Winchester, Connecticut

File Name : 18223 Site Code : 18223 Start Date : 12/5/2018

Page No : 1

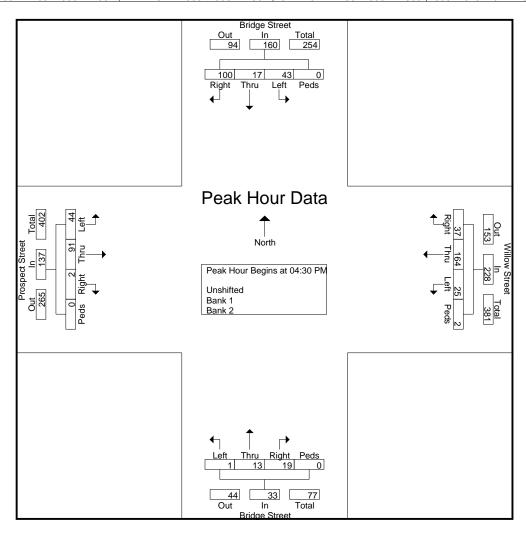
Groups Printed- Unshifted - Bank 1 - Bank 2

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Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:00 PM	11	3	10	0	24	13	46	8	0	67	7	5	0	0	12	0	22	10	0	32	135
04:15 PM	19	2	6	0	27	4	46	4	0	54	4	1	0	0	5	0	25	8	0	33	119
04:30 PM	29	4	14	0	47	8	47	7	0	62	2	4	1	0	7	0	25	12	0	37	153
04:45 PM	18	7	12	0	37	10	44	11	1	66	2	2	0	0	4	0	25	11	0	36	143
Total	77	16	42	0	135	35	183	30	1	249	15	12	1	0	28	0	97	41	0	138	550
05:00 PM	20	4	11	0	35	12	39	4	0	55	15	6	0	0	21	1	18	11	0	30	141
05:15 PM	33	2	6	0	41	7	34	3	1	45	0	1	0	0	1	1	23	10	0	34	121
05:30 PM	37	5	7	0	49	9	36	3	0	48	1	0	1	0	2	0	36	15	0	51	150
05:45 PM	11	3	7	0	21	4	33	5	0	42	3	1	0	0	4	0	25	13	0	38	105
Total	101	14	31	0	146	32	142	15	1	190	19	8	1	0	28	2	102	49	0	153	517
<b>Grand Total</b>	178	30	73	0	281	67	325	45	2	439	34	20	2	0	56	2	199	90	0	291	1067
Apprch %	63.3	10.7	26	0		15.3	74	10.3	0.5		60.7	35.7	3.6	0		0.7	68.4	30.9	0		
Total %	16.7	2.8	6.8	0	26.3	6.3	30.5	4.2	0.2	41.1	3.2	1.9	0.2	0	5.2	0.2	18.7	8.4	0	27.3	
Unshifted	178	30	72	0	280	66	325	45	2	438	34	20	2	0	56	2	198	89	0	289	1063
% Unshifted																					
Bank 1	0	0	1	0	1	1	0	0	0	1	0	0	0	0	0	0	1	1	0	2	4
% Bank 1	0	0	1.4	0	0.4	1.5	0	0	0	0.2	0	0	0	0	0	0	0.5	1.1	0	0.7	0.4
Bank 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Kensington, Connecticut 06037 (860) 828-1693

> File Name : 18223 Site Code : 18223 Start Date : 12/5/2018

			dge St					llow S					dge S om So					spect :	Street		
Start Time	Dialet	Thru	Left			Diaht	Thru	Left	Peds		Diaht	Thru	Left	Peds		Diaht	Thru	Left	Peds		
	Right				App. Total	Right			Peus	App. Total	Right	HIIIU	Leit	Peus	App. Total	Right	HIIIU	Leit	Peus	App. Total	Int. Total
Peak Hour A	nalysi	s Fron	า 04:00	DPM to	o 05:45	PM - I	Peak 1	of 1													
Peak Hour fo	or Enti	re Inte	rsectio	n Begi	ins at 0	4:30 P	M														
04:30 PM	29	4	14	0	47	8	47	7	0	62	2	4	1	0	7	0	25	12	0	37	153
04:45 PM	18	7	12	0	37	10	44	11	1	66	2	2	0	0	4	0	25	11	0	36	143
05:00 PM	20	4	11	0	35	12	39	4	0	55	15	6	0	0	21	1	18	11	0	30	141
05:15 PM	33	2	6	0	41	7	34	3	1_	45	0	1	0	0	1	1	23	10	0	34	121
Total Volume	100	17	43	0	160	37	164	25	2	228	19	13	1	0	33	2	91	44	0	137	558
% App. Total	62.5	10.6	26.9	0		16.2	71.9	11	0.9		57.6	39.4	3	0		1.5	66.4	32.1	0		
PHF	.758	.607	.768	.000	.851	.771	.872	.568	.500	.864	.317	.542	.250	.000	.393	.500	.910	.917	.000	.926	.912

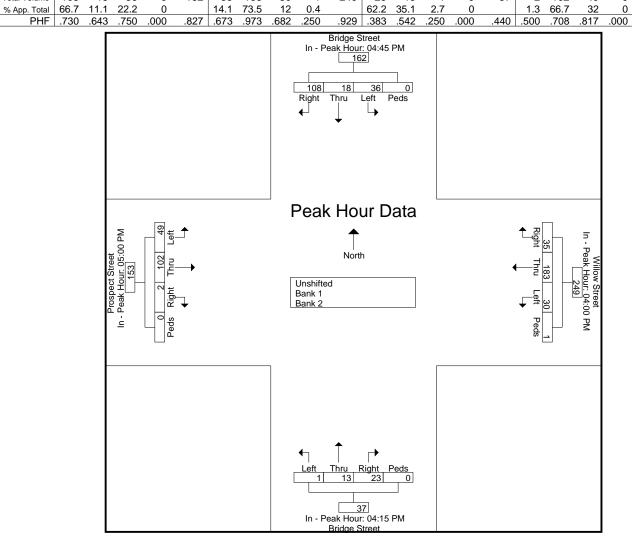


Kensington, Connecticut 06037 (860) 828-1693

File Name : 18223 Site Code : 18223 Start Date : 12/5/2018

.750

			dge St					low S					dge St					•	Street		
		<u> </u>	om No	ortn				<u>rom E</u>	ast			FI	om Sc	<u>utn</u>				rom W	est		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour A	nalysis	From	า 04:00	OPM to	o 05:45	PM - I	Peak 1	of 1													
Peak Hour fo	or Each	Appr	oach [	<b>Begins</b>	at:																,
	04:45 PM					04:00 PM	1				04:15 PM					05:00 PM	1				
+0 mins.	18	7	12	0	37	13	46	8	0	67	4	1	0	0	5	1	18	11	0	30	
+15 mins.	20	4	11	0	35	4	46	4	0	54	2	4	1	0	7	1	23	10	0	34	
+30 mins.	33	2	6	0	41	8	47	7	0	62	2	2	0	0	4	0	36	15	0	51	
+45 mins.	37	5	7	0	49	10	44	11	1_	66	15	6	0	0	21	0	25	13	0	38	
Total Volume	108	18	36	0	162	35	183	30	1	249	23	13	1	0	37	2	102	49	0	153	



Kensington, Connecticut 06037 (860) 828-1693

Bridge Street at Depot Street Winchester, Connecticut

File Name: 18224 Site Code: 18224

Start Date : 12/5/2018

Page No : 1

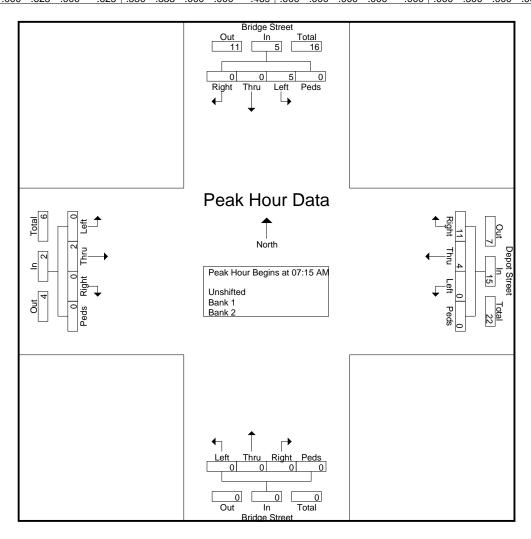
Groups Printed- Unshifted - Bank 1 - Bank 2

											Unshiii	<u>ieu - D</u>		- Dan								
			Bri	dge S	treet			De	pot St	reet			Bri	dge S	treet							
			Fı	om N	orth			F	rom E	ast				om So				Fr	om W	est /		
St	art Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07	7:00 AM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
07	7:15 AM	0	0	1	0	1	2	1	0	0	3	0	0	0	0	0	0	1	0	0	1	5
07	7:30 AM	0	0	2	0	2	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	5
07	7:45 AM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
	Total	0	0	4	0	4	6	1	0	0	7	0	0	0	0	0	0	1	0	0	1	12
	3:00 AM	0	0	2	0	2	5	3	0	0	8	0	0	0	0	0	0	1	0	0	1	11
	3:15 AM	0	0	1	0	1	1	1	0	0	2	0	0	0	0	0	0	1	0	0	1	4
***	BREAK *	**																				
	Total	0	0	3	0	3	6	4	0	0	10	0	0	0	0	0	0	2	0	0	2	15
	and Total	0	0	7	0	7	12	5	0	0	17	0	0	0	0	0	0	3	0	0	3	27
Αŗ	oprch %	0	0	100	0		70.6	29.4	0	0		0	0	0	0		0	100	0	0		
	Total %	0	0	25.9	0	25.9	44.4	18.5	0	0	63	0	0	0	0	0	0	11.1	0	0	11.1	
U	Inshifted	0	0	7	0	7	12	4	0	0	16	0	0	0	0	0	0	3	0	0	3	26
_%	Unshifted																					
	Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
_%	Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	Bank 2	0	0	0	0	0	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0	1
%	Bank 2	0	0	0	0	0	0	20	0	0	5.9	0	0	0	0	0	0	0	0	0	0	3.7

Kensington, Connecticut 06037 (860) 828-1693

> File Name : 18224 Site Code : 18224 Start Date : 12/5/2018

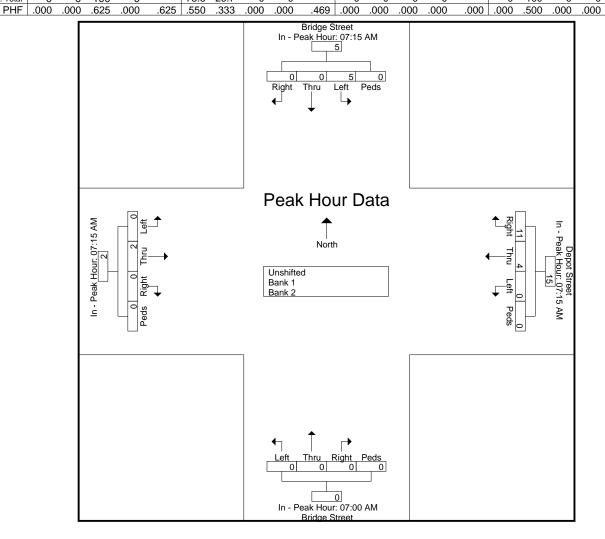
		Bri	dge S	treet			De	pot St	reet			Bri	dge S	treet							
		Fr	om No	orth			F	rom E	ast			Fr	om So	outh			Fı	rom W	est /		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour A	nalysi	s From	า 07:00	O AM to	o 08:45	AM - I	Peak 1	of 1													
Peak Hour fo	or Enti	re Inte	rsectio	n Beg	ins at 0	7:15 A	M														
07:15 AM	0	0	1	0	1	2	1	0	0	3	0	0	0	0	0	0	1	0	0	1	5
07:30 AM	0	0	2	0	2	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	5
07:45 AM	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	1
08:00 AM	0	0	2	0	2	5	3	0	0	8	0	0	0	0	0	0	1	0	0	1	11
Total Volume	0	0	5	0	5	11	4	0	0	15	0	0	0	0	0	0	2	0	0	2	22
% App. Total	0	0	100	0		73.3	26.7	0	0		0	0	0	0		0	100	0	0		
PHF	.000	.000	625	.000	625	550	.333	.000	.000	469	.000	.000	.000	.000	.000	.000	500	.000	.000	.500	.500



Kensington, Connecticut 06037 (860) 828-1693

File Name : 18224 Site Code : 18224 Start Date : 12/5/2018

			dge St					pot St					dge S					10	, .		
		⊢r	om No	ortn			F	<u>rom E</u>	<u>ast</u>			<u> </u>	om Sc	uth			<u> Fr</u>	om W	est		
Start Time	Right T	hru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. T
Peak Hour A	nalysis	From	า 07:00	O AM to	o 08:45	AM - I	Peak 1	of 1													
Peak Hour fo	or Each	Appr	oach I	Begins	at:																,
	07:15 AM					07:15 AM	1				07:00 AM					07:15 AM	I				
+0 mins.	0	0	1	0	1	2	1	0	0	3	0	0	0	0	0	0	1	0	0	1	
+15 mins.	0	0	2	0	2	3	0	0	0	3	0	0	0	0	0	0	0	0	0	0	
+30 mins.	0	0	0	0	0	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
+45 mins.	0	0	2	0	2	5	3	0	0	8	0	0	0	0	0	0	1_	0	0	1	
Total Volume	0	0	5	0	5	11	4	0	0	15	0	0	0	0	0	0	2	0	0	2	
% App. Total	0	0	100	0		73.3	26.7	0	0		0	0	0	0		0	100	0	0		



Kensington, Connecticut 06037 (860) 828-1693

Bridge Street at Depot Street Winchester, Connecticut

File Name: 18225 Site Code: 18225 Start Date : 12/5/2018

Page No : 1

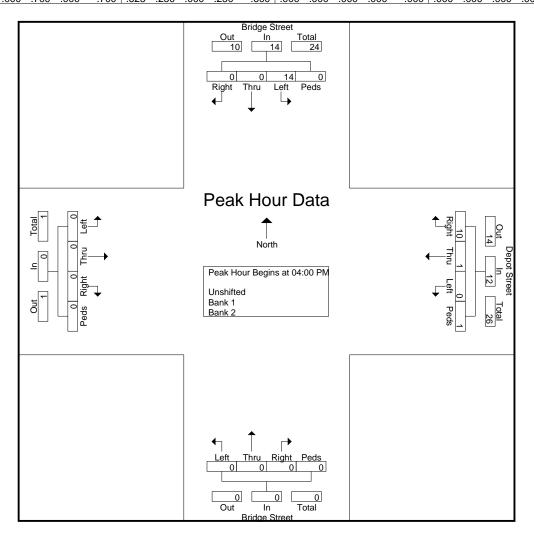
Groups Printed- Unshifted - Bank 1 - Bank 2

		Bri	dge S	treet				pot St		01101111	<u> </u>	Bri	dge S	treet							
			om No				F	rom E	ast				om So				Fr	om W	est est		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:00 PM	0	0	4	0	4	3	1	0	0	4	0	0	0	0	0	0	0	0	0	0	8
04:15 PM	0	0	3	0	3	4	0	0	1	5	0	0	0	0	0	0	0	0	0	0	8
04:30 PM	0	0	2	0	2	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3
04:45 PM	0	0	5	0	5	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	7
Total	0	0	14	0	14	10	1	0	1	12	0	0	0	0	0	0	0	0	0	0	26
						ı															ı
05:00 PM	0	0	1	0	1	5	1	0	0	6	0	0	0	0	0	0	0	0	0	0	7
*** BREAK *	**																				ı
05:30 PM	0	0	1	0	1	0	2	0	0	2	0	0	0	0	0	0	1	0	0	1	4
05:45 PM	0	0	1	0	1	1	1	0	0	2	0	0	0	0	0	0	0	0	0	0	3
Total	0	0	3	0	3	6	4	0	0	10	0	0	0	0	0	0	1	0	0	1	14
						ı															ı
Grand Total	0	0	17	0	17	16	5	0	1	22	0	0	0	0	0	0	1	0	0	1	40
Apprch %	0	0	100	0		72.7	22.7	0	4.5		0	0	0	0		0	100	0	0		
Total %	0	0	42.5	0	42.5	40	12.5	0	2.5	55	0	0	0	0	0	0	2.5	0	0	2.5	
Unshifted	0	0	17	0	17	16	5	0	1	22	0	0	0	0	0	0	1	0	0	1	40
% Unshifted																					
Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 1	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0_
Bank 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Bank 2	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

Kensington, Connecticut 06037 (860) 828-1693

> File Name : 18225 Site Code : 18225 Start Date : 12/5/2018

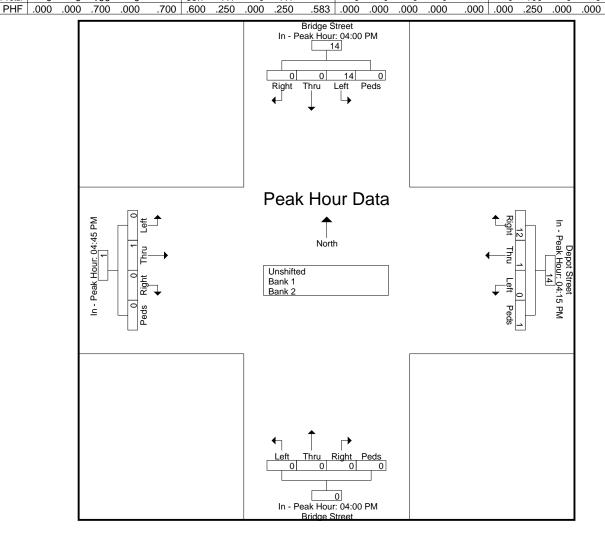
		Bri	dge S	treet			De	pot St	treet			Bri	dge S	treet							
		Fr	om No	orth			F	rom E	ast			Fr	om So	outh			Fı	rom W	/est		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
Peak Hour A	nalysi	s Fron	า 04:00	PM to	o 05:45	PM - I	Peak 1	of 1													
Peak Hour fo	or Enti	re Inte	rsection	n Beg	ins at 0	4:00 P	M														
04:00 PM	0	0	4	0	4	3	1	0	0	4	0	0	0	0	0	0	0	0	0	0	8
04:15 PM	0	0	3	0	3	4	0	0	1	5	0	0	0	0	0	0	0	0	0	0	8
04:30 PM	0	0	2	0	2	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	3
04:45 PM	0	0	5	0	5	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	7
Total Volume	0	0	14	0	14	10	1	0	1	12	0	0	0	0	0	0	0	0	0	0	26
% App. Total	0	0	100	0		83.3	8.3	0	8.3		0	0	0	0		0	0	0	0		
PHF	.000	.000	.700	.000	.700	625	250	.000	250	.600	.000	.000	.000	.000	.000	.000	.000	.000	.000	.000	.813



Kensington, Connecticut 06037 (860) 828-1693

File Name : 18225 Site Code : 18225 Start Date : 12/5/2018

		D:	-l O								I	D.:	-1 0								1
			dge St					pot St					dge S								
		Fr	om No	orth			F	<u>rom E</u>	ast			Fr	om So	outh			Fr	om W	/est		
Start Time	Right 1	Γhru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int.
Peak Hour A	Analysis	From	า 04:00	OPM to	05:45	PM - F	Peak 1	of 1													
Peak Hour f	or Each	Appr	oach E	<b>Begins</b>	at:																_
	04:00 PM					04:15 PM					04:00 PM					04:45 PM	1				
+0 mins.	0	0	4	0	4	4	0	0	1	5	0	0	0	0	0	0	0	0	0	0	
+15 mins.	0	0	3	0	3	1	0	0	0	1	0	0	0	0	0	0	0	0	0	0	
+30 mins.	0	0	2	0	2	2	0	0	0	2	0	0	0	0	0	0	0	0	0	0	
+45 mins.	0	0	5	0	5	5	1_	0	0	6	0	0	0	0	0	0	1	0	0	1	
Total Volume	0	0	14	0	14	12	1	0	1	14	0	0	0	0	0	0	1	0	0	1	
% App. Total	0	0	100	0		85.7	7.1	0	7.1		0	0	0	0		0	100	0	0		



# **Intersection Capacity Analysis**

Intersection												
Int Delay, s/veh	6											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	211	2	1	0	4	11	2	42	0	5	20	176
Future Vol, veh/h	211	2	1	0	4	11	2	42	0	5	20	176
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	e,# -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	84	84	84	47	47	47	81	81	81	70	70	70
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	251	2	1	0	9	23	2	52	0	7	29	251
Major/Minor	Minor2			Minor1			Major1			Major2		
Conflicting Flow All	241	225	155	226	350	52	280	0	0	52	0	0
Stage 1	169	169	-	56	56	-	00	-	-	-	-	-
Stage 2	72	56	_	170	294	_	_	_	_	_	_	_
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	_	_
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	_		-	_
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	_	_	-	_	_	_	_
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	_	2.218	-	_
Pot Cap-1 Maneuver	713	674	891	729	574	1016	1283	-	-	1554	-	-
Stage 1	833	759	-	956	848	-	-	_	_	-	_	_
Stage 2	938	848	-	832	670	-	-	-	-	-	-	-
Platoon blocked, %								-	-		-	-
Mov Cap-1 Maneuver	684	669	891	722	569	1016	1283	-	-	1554	-	_
Mov Cap-2 Maneuver	684	669	-	722	569	-	-	-	-	-	-	-
Stage 1	831	754	-	954	846	-	-	-	-	-	-	-
Stage 2	905	846	-	823	666	-	-	-	-	-	-	-
<b>J</b> -												
Approach	EB			WB			NB			SB		
HCM Control Delay, s	13.3			9.5			0.4			0.2		
HCM LOS	В			A								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBL n1	SBL	SBT	SBR			
Capacity (veh/h)		1283	-	-	685	840	1554	-				
HCM Lane V/C Ratio		0.002	_		0.372			_	_			
HCM Control Delay (s)		7.8	0	_	13.3	9.5	7.3	0	_			
HCM Lane LOS		Α.	A	_	В	Α.	Α.	A	_			
HCM 95th %tile Q(veh	)	0	-	-	1.7	0.1	0	-	_			
7000 00 000	,					0.1						

Synchro 9 Report Page 1 Baseline

Intersection						
Int Delay, s/veh	3.3					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	*y*		ĵ.			4
Traffic Vol, veh/h	86	29	69	195	52	125
Future Vol, veh/h	86	29	69	195	52	125
Conflicting Peds, #/hr	0	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	_		_	
Storage Length	0	-	-	-	-	-
Veh in Median Storage		_	0	_	_	0
Grade, %	0	_	0	_	-	0
Peak Hour Factor	92	92	81	81	70	70
Heavy Vehicles, %	2	2	2	2	2	2
Mymt Flow	93	32	85	241	74	179
WWITELLOW	55	02	00	271	17	175
Major/Minor I	Minor1	N	Major1	N	Major2	
Conflicting Flow All	533	206	0	0	326	0
Stage 1	206	-	-	-	-	-
Stage 2	327	-	-	-	-	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	507	835	-	-	1234	-
Stage 1	829	-	-	-	-	-
Stage 2	731	-	-	-	-	-
Platoon blocked, %			-	_		-
Mov Cap-1 Maneuver	473	835	_	-	1234	-
Mov Cap-2 Maneuver	473	-	_	_	_	_
Stage 1	829	-	_	-	_	_
Stage 2	682	_	_	_	_	_
Olago Z	002					
Approach	WB		NB		SB	
HCM Control Delay, s	13.9		0		2.4	
HCM LOS	В					
Minor Lanc/Major Mum	.+	NBT	NDDV	MDI 51	SBL	SBT
Minor Lane/Major Mvm	l			VBLn1		
Capacity (veh/h)		-	-	•••	1234	-
HCM Control Dalace (a)		-		0.235	0.06	-
HCM Control Delay (s)		-	-		8.1	0
HCM Lane LOS HCM 95th %tile Q(veh)		-	-	В	0.2	Α
HUM 45th %tile ()(veh)		_	-	0.9	117	-

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Intersection												
Int Delay, s/veh	3.8											
Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	135	1	2	0	4	11	1	20	0	14	28	264
Future Vol, veh/h	135	1	2	0	4	11	1	20	0	14	28	264
Conflicting Peds, #/hr	0	0	0	0	0	0	0	0	0	0	0	0
Sign Control	Stop	Stop	Stop	Stop	Stop	Stop	Free	Free	Free	Free	Free	Free
RT Channelized	-	-	None	-	-	None	-	-	None	-	-	None
Storage Length	-	-	-	-	-	-	-	-	-	-	-	-
Veh in Median Storage	, # -	0	-	-	0	-	-	0	-	-	0	-
Grade, %	-	0	-	-	0	-	-	0	-	-	0	-
Peak Hour Factor	93	93	93	60	60	60	39	39	39	85	85	85
Heavy Vehicles, %	2	2	2	2	2	2	2	2	2	2	2	2
Mvmt Flow	145	1	2	0	7	18	3	51	0	16	33	311
Major/Minor	Minor2			Minor1			Major1		1	Major2		
Conflicting Flow All	291	278	189	279	433	51	344	0	0	51	0	0
Stage 1	221	221	-	57	57	-	-	-	-	-	_	-
Stage 2	70	57	-	222	376	-	-	-	_	-	-	-
Critical Hdwy	7.12	6.52	6.22	7.12	6.52	6.22	4.12	-	-	4.12	-	-
Critical Hdwy Stg 1	6.12	5.52	-	6.12	5.52	-	-	-	_	_	-	-
Critical Hdwy Stg 2	6.12	5.52	-	6.12	5.52	-	-	-	-	-	-	-
Follow-up Hdwy	3.518	4.018	3.318	3.518	4.018	3.318	2.218	-	-	2.218	-	_
Pot Cap-1 Maneuver	661	630	853	673	516	1017	1215	-	-	1555	_	-
Stage 1	781	720	-	955	847	-	-	-	_	-	-	-
Stage 2	940	847	-	780	616	-	-	-	-	-	-	-
Platoon blocked, %								-	_		-	-
Mov Cap-1 Maneuver	635	620	853	662	508	1017	1215	-	-	1555	-	-
Mov Cap-2 Maneuver	635	620	-	662	508	-	-	-	_	-	-	-
Stage 1	779	711	-	952	844	-	-	-	-	-	-	-
Stage 2	913	844	_	767	608	-	_	_	_	_	_	_
-					2.3							
Approach	EB			WB			NB			SB		
HCM Control Delay, s	12.4			9.6			0.4			0.3		
HCM LOS	В			A								
Minor Lane/Major Mvm	nt	NBL	NBT	NBR	EBLn1V	VBLn1	SBL	SBT	SBR			
Capacity (veh/h)		1215	-	-	637	803	1555	-	-			
HCM Lane V/C Ratio		0.002	_	_	0.233			_	_			
HCM Control Delay (s)		8	0	_	12.4	9.6	7.3	0	_			
HCM Lane LOS		A	A	_	В	A	A	A	_			
HCM 95th %tile Q(veh)	)	0	-	-	0.9	0.1	0	-	-			
7,5000 24(1,500)												

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Intersection						
Int Delay, s/veh	6.1					
Movement	WBL	WBR	NBT	NBR	SBL	SBT
Lane Configurations	WBL	אטא		NDK	ODL	<u> </u>
Traffic Vol, veh/h	189	37	<b>1</b> → 57	110	43	<b>식</b> 117
Future Vol, veh/h	189	37	57 57	110	43	117
	109	0	0	0	43	0
Conflicting Peds, #/hr						Free
Sign Control	Stop	Stop	Free	Free	Free	
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		-	0	-	-	0
Grade, %	0	-	0	-	-	0
Peak Hour Factor	86	86	39	39	85	85
Heavy Vehicles, %	2	2	2	2	2	2
Mvmt Flow	220	43	146	282	51	138
Major/Minor	Minor1		Major1		Major2	
Conflicting Flow All	527	287	0	0	428	0
Stage 1	287	-	-	-	-	-
Stage 2	240	-	-	-	<u>-</u>	-
Critical Hdwy	6.42	6.22	-	-	4.12	-
Critical Hdwy Stg 1	5.42	-	-	-	-	-
Critical Hdwy Stg 2	5.42	-	-	-	-	-
Follow-up Hdwy	3.518	3.318	-	-	2.218	-
Pot Cap-1 Maneuver	512	752	-	-	1131	-
Stage 1	762	-	-	-	-	-
Stage 2	800	-	-	-	-	-
Platoon blocked, %			-	-		-
Mov Cap-1 Maneuver	487	752	-	-	1131	_
Mov Cap-2 Maneuver	487	-	-	_	_	-
Stage 1	762	_	_	_	_	_
Stage 2	761	_	_	_	_	_
Olage 2	701					
Approach	WB		NB		SB	
HCM Control Delay, s	18.9		0		2.2	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBT	NRDV	VBLn1	SBL	SBT
	IL		NDKV			SDI
Capacity (veh/h)		-	-	517	1131	-
HCM Lane V/C Ratio		-		0.508		-
HCM Control Delay (s)		-	-	18.9	8.3	0
HCM Lane LOS		-	-	С	Α	Α
HCM 95th %tile Q(veh	)	-	-	2.8	0.1	-

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